

Prefabricated High-Strength Rebar Systems with High-Performance Concrete for Accelerated Construction of Nuclear Concrete Structures



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DOE-NE NEET-1 Program Goals

- Nuclear Energy Enabling Technologies Program-Advanced Methods for Manufacturing (NEET-1)
- “Accelerate innovations that reduce the cost and schedule of constructing new nuclear plants and make fabrication of nuclear power plant components faster, cheaper, and more reliable.”
- “Develop new/revised nuclear industry codes and standards that enable the utilization of newly developed technologies.”



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Project Objective

Reduce field construction times and fabrication costs of reinforced concrete nuclear structures through:

- 1) High-strength reinforcing steel (rebar)
- 2) Prefabricated rebar assemblies, including headed anchorages
- 3) High-strength concrete

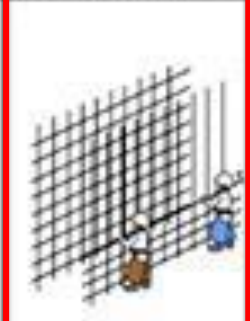

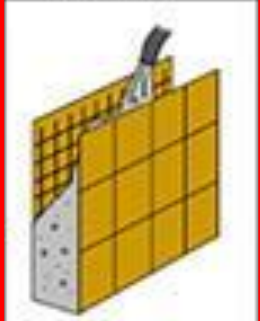
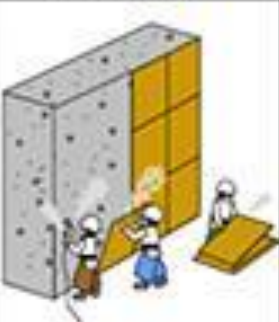
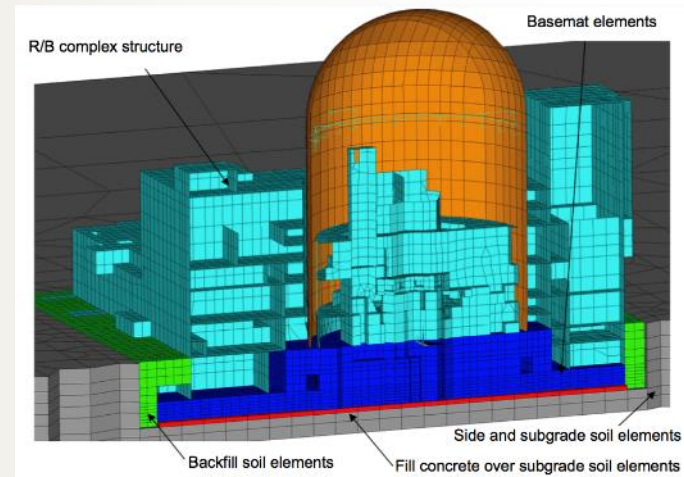
Work Structure	Rebar arrangement	Form work (assembling)	Placing concrete	Form work (removal)
RC				
28days	13days	7days	4days	4days

Figure From:
MPR-2610 Rev 2
Sept. 2004



Project Scope

- Explore effectiveness, code conformity, and viability of existing high-strength materials
- Focus on stocky shear walls – predominant load resisting members in nuclear structures (pressure vessels not in scope)
- Aim to reduce complexities in rebar to improve construction quality and ease of inspection

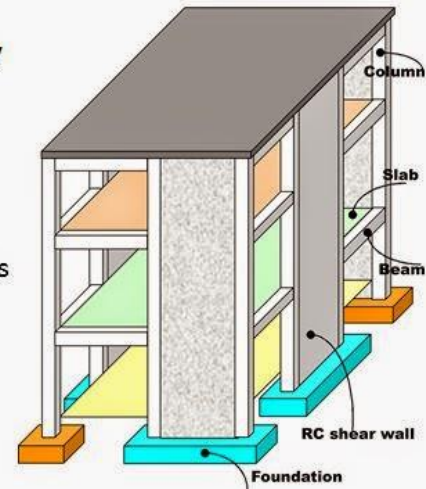


US-APWR Design Control Doc.

Project Scope

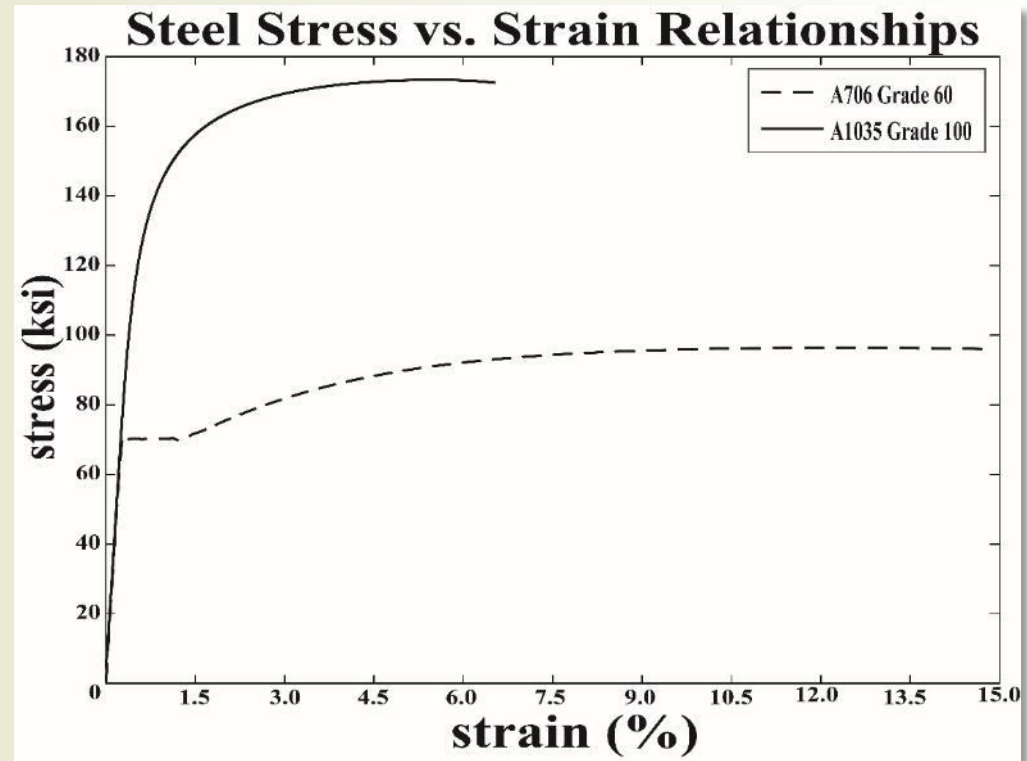
- Explore effectiveness, code conformity, and viability of existing high-strength materials
- Focus on stocky shear walls – predominant load resisting members in nuclear structures (pressure vessels not in scope)
- Aim to reduce complexities in rebar to improve construction quality and ease of inspection

RC shear walls carry earthquake loads down to the foundation. They provide large strength and stiffness to buildings in the direction of their orientation.



High-Strength Materials

- High-strength rebar (up to Grade 120) with high-strength concrete (up to 20,000 psi compressive strength)
- ACI 349 limits headed bars and shear reinforcement to Grade 60
- Concrete strength of 5,000 psi typical in current practice



Potential Benefits

**Most Congested
(current)**

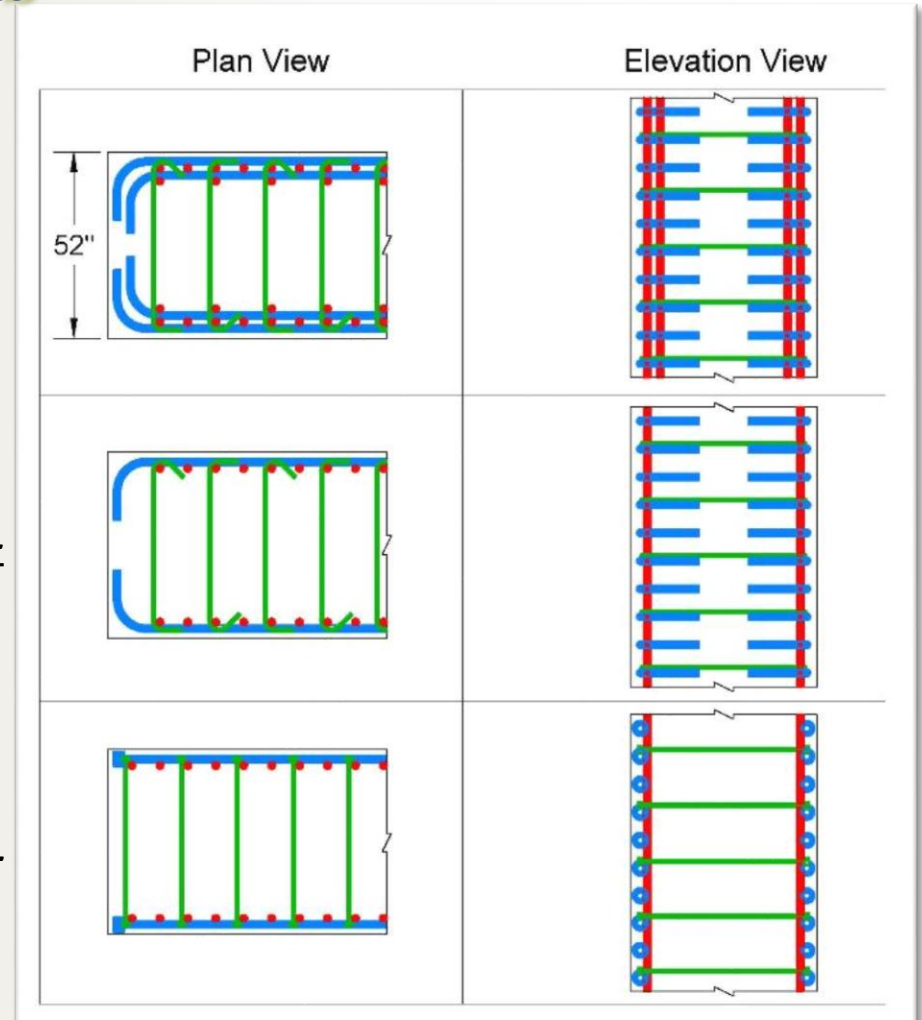


*Multiple layers
of hooked
Grade 60 bars*

*Fewer layers
of hooked high-
strength bars*

**Least Congested
(envisioned)**

*Fewer layers
of headed high-
strength bars*



Collaboration



UNIVERSITY OF
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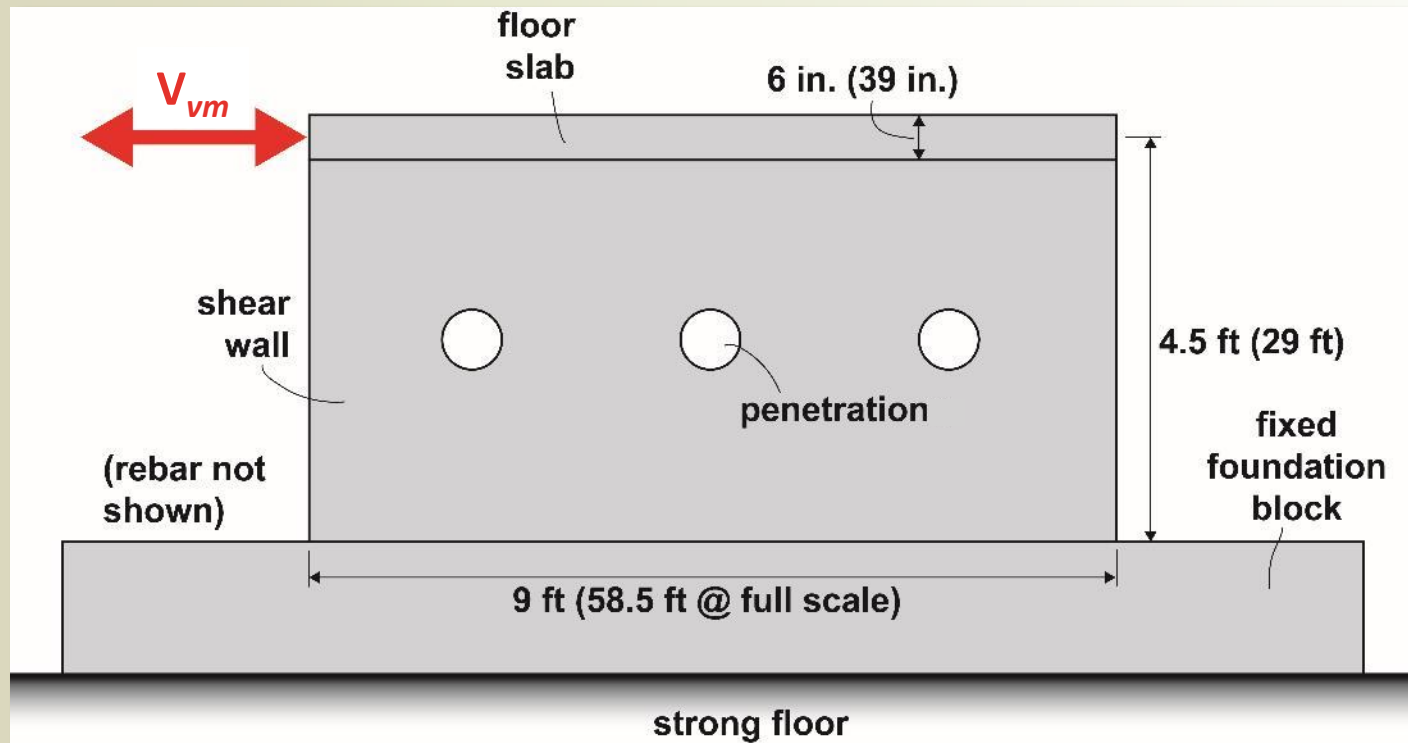


Outline

1. Numerical Modeling
2. Limit-Benefit Analysis
3. Cost-Benefit Analysis
4. Experimental Testing

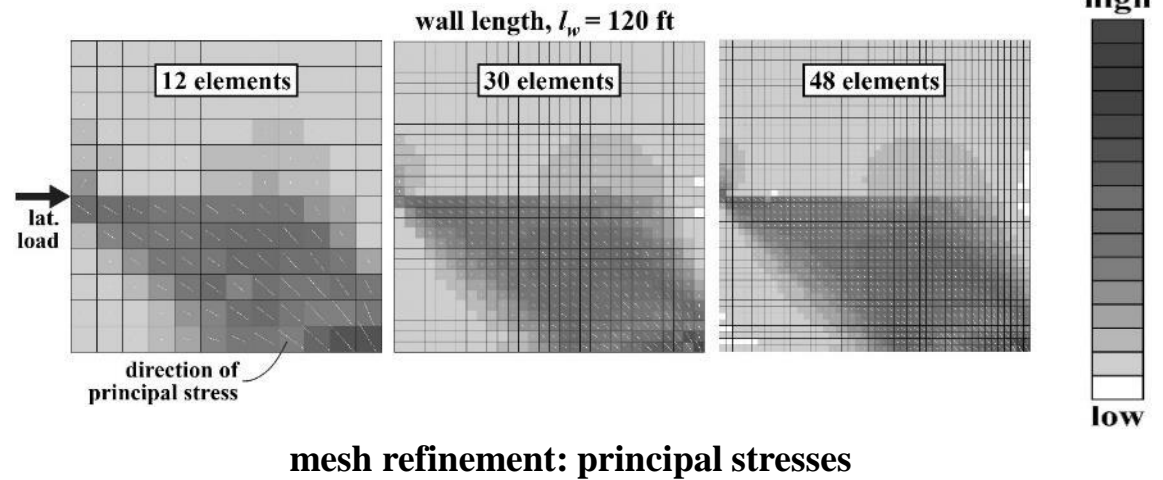
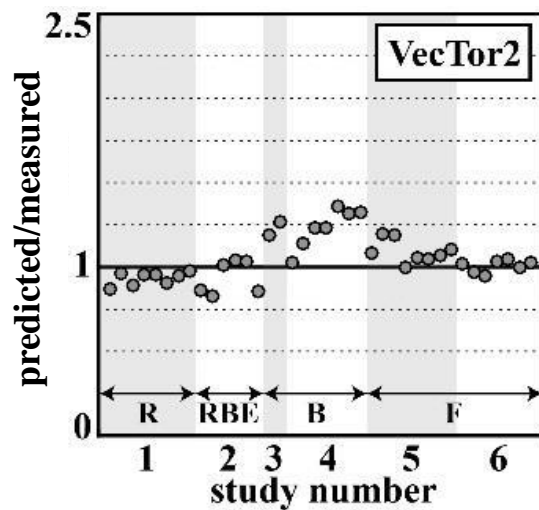
1. Modeling Approach

- Evaluated methods for predicting peak lateral strength (V_{vm}) of stocky shear walls:
 - 1) Closed-form Design Methods
 - 2) Finite Element Modeling Predictions



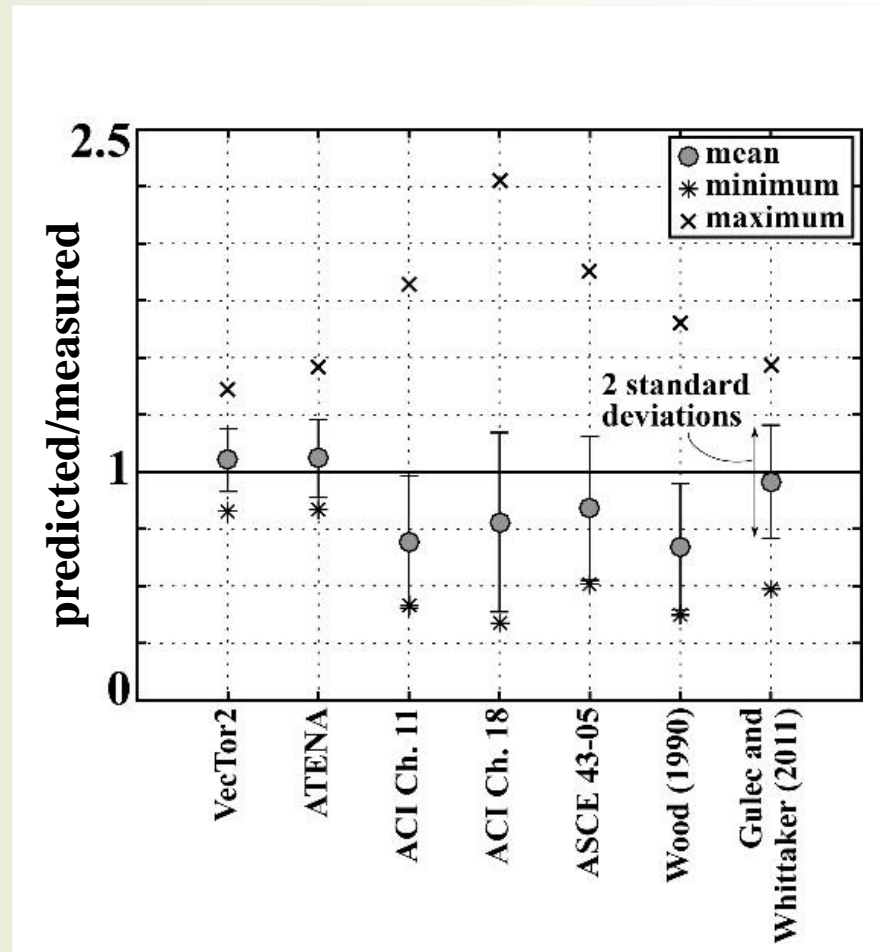
1. VecTor2 Finite Element Model

- Reliably captures the peak strength for rectangular walls with a wide range of material properties and base moment-to-shear ratios



1. Comparison of Predictions

- Design equations should be revisited, although mean predictions are conservative, there are unconservative outliers for typical nuclear wall geometries.
- VecTor2 and ATENA are reliable for predicting peak strength; ABAQUS will also be used.



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2. Limit-Benefit Analysis

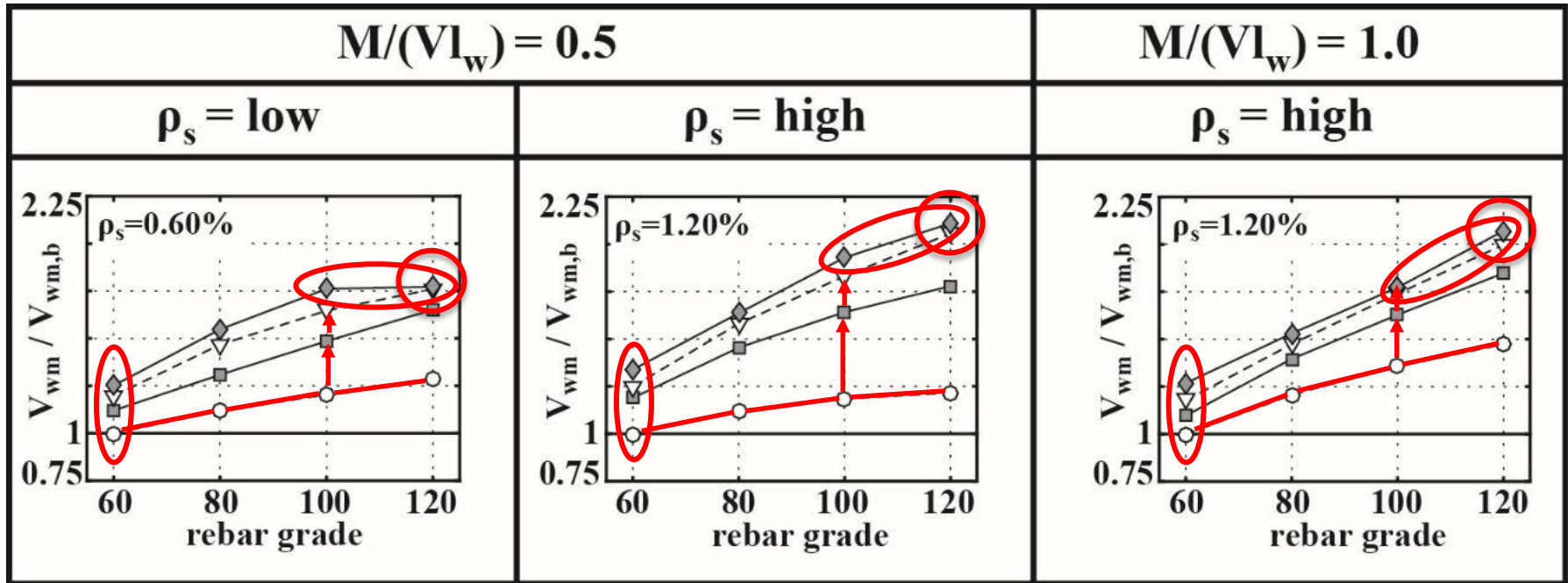
Numerical limit-benefit study to establish effects of high-strength materials on peak lateral strength of low-aspect-ratio shear walls:

- Parametric numerical investigation of 192 walls
- Peak strength predicted via VecTor2 finite element model

Parameter	Wall 1	Wall 2	Wall 3
length, l_w (ft)	20	60	120
height, h_w (ft)	40	120	120
thickness, t_w (in.)	15	45	45
moment to shear ratio, $M/(Vl_w)$	0.5, 1.0	0.5, 1.0	0.5, 1.0
concrete strength, f'_c (ksi)	5, 10, 15, 20	5, 10, 15, 20	5, 10, 15, 20
rebar strength, f_y (ksi)	60, 80, 100, 120	60, 80, 100, 120	60, 80, 100, 120
reinforcement ratio, ρ_s (%)	0.25, 0.50	0.60, 1.20	0.60, 1.20

2. Representative Results

Wall 2 (60 ft long, 120 ft tall, 45 in. thick):



-- \circ -- $f'_c = 5.00$ ksi

-- \square -- $f'_c = 10.0$ ksi

-- ∇ -- $f'_c = 15.0$ ksi

-- \diamond -- $f'_c = 20.0$ ksi

V_{wm} = Predicted peak lateral strength

$V_{wm,b}$ = Predicted peak lateral strength of “benchmark” with normal strength materials

2. Limit-Benefit Summary

- Combination of high-strength rebar with high-strength concrete resulted in a higher-performing structure than with either high-strength material on its own
- Greatest benefits of high-strength materials for walls with large rebar ratios, ρ_s
- Significant benefits by using concrete strength of $f'_c = 10$ ksi, with diminishing returns for higher strengths
- Rebar strength becomes more important and concrete strength becomes less important as $M/(Vl_w)$ ratio is increased

Outline

1. Numerical Modeling
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3. Cost-Benefit Analysis

- Numerical cost-benefit study of economic effectiveness of high-strength materials for low-rise shear walls:
 - Parametric numerical investigation of 2304 walls

Parameter	Wall 1	Wall 2	Wall 3
length, l_w (ft)	20	60	120
height, h_w (ft)	40	120	120
thickness, t_w (in.)	10, 15 , 20	30, 45 , 60	30, 45 , 60
moment to shear ratio, $M/(Vl_w)$	0.5 , 1.0	0.5 , 1.0	0.5 , 1.0
concrete strength, f'_c (ksi)	5 , 10, 15, 20	5 , 10, 15, 20	5 , 10, 15, 20
rebar strength, f_y (ksi)	60 , 80, 100, 120	60 , 80, 100, 120	60 , 80, 100, 120
reinforcement ratio, ρ_l (%)	low to high	low to high	low to high
ratio of reinforcement, ρ_t/ρ_l	0.80, 1.00	0.80, 1.00	0.80, 1.00

3. Construction Cost Metric

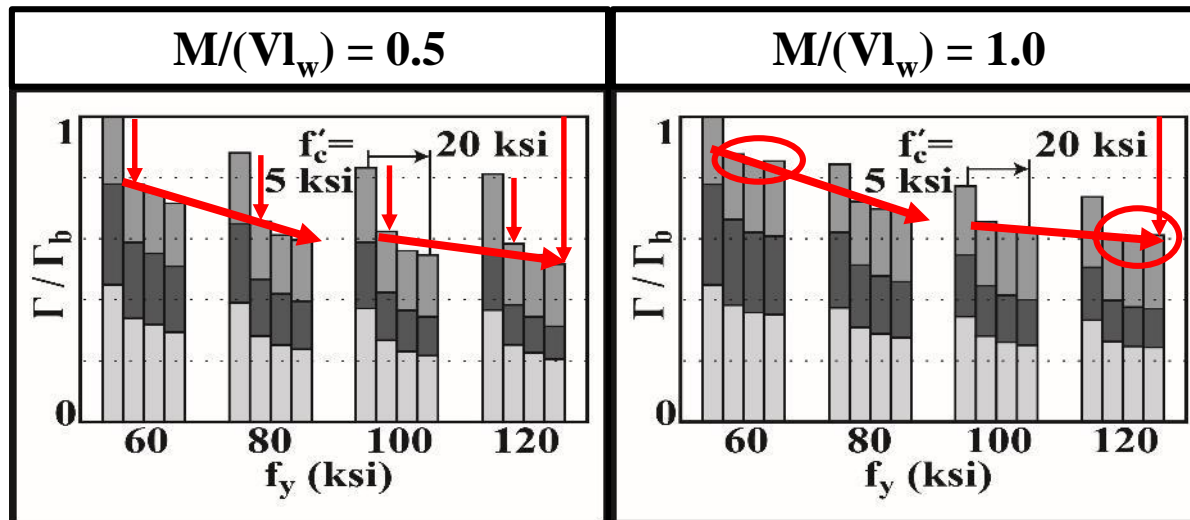
- Construction cost metric (Γ) includes rebar material cost, rebar labor cost, and concrete material cost (C_w), normalized by peak strength (V_{wm}):

$$\Gamma = \frac{C_w}{V_{wm}}$$

- Γ then normalized by “benchmark” Γ_b for walls with normal-strength materials

3. Construction Cost Metric Results

Wall 2 (60 ft long, 120 ft tall, 45 in. thick), $\rho_l = \text{very high}$:



rebar (material)
 rebar (labor)
 concrete (material)

$$\Gamma = \frac{C_w}{V_{wm}}$$

Γ = Construction cost metric

Γ_b = Construction cost metric of “benchmark” with normal-strength materials

C_w = Total cost of rebar material, rebar labor, and concrete material

V_{wm} = Predicted peak lateral strength

3. Cost-Benefit Summary

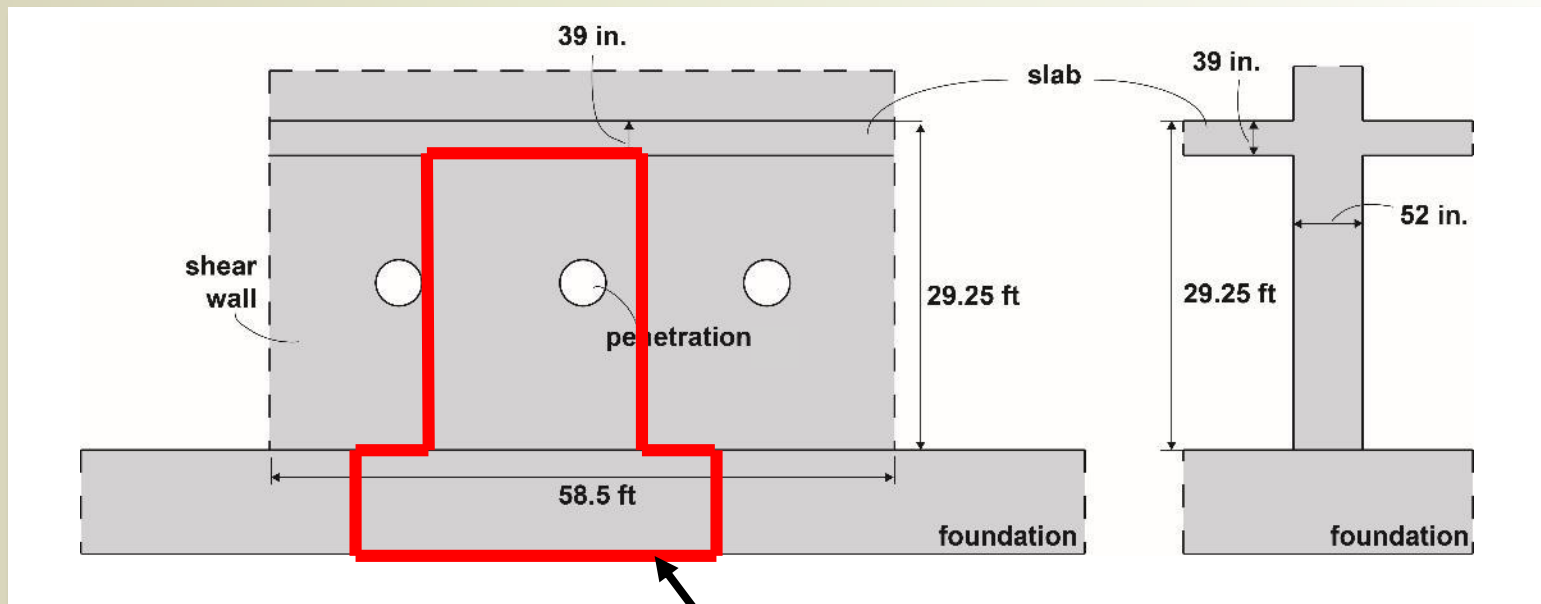
- Combination of high-strength rebar with high-strength concrete resulted in greatest economic benefits, especially for walls with lower $M/(Vl_w)$ ratios and large reinforcement ratios, ρ_s
- A concrete strength of $f'_c = 10$ ksi showed the largest incremental reduction in construction cost; higher concrete strengths can increase normalized cost metric
- Rebar grades greater than 100 can lead to negligible economic benefits due to the increased unit cost

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4. Experimental Testing

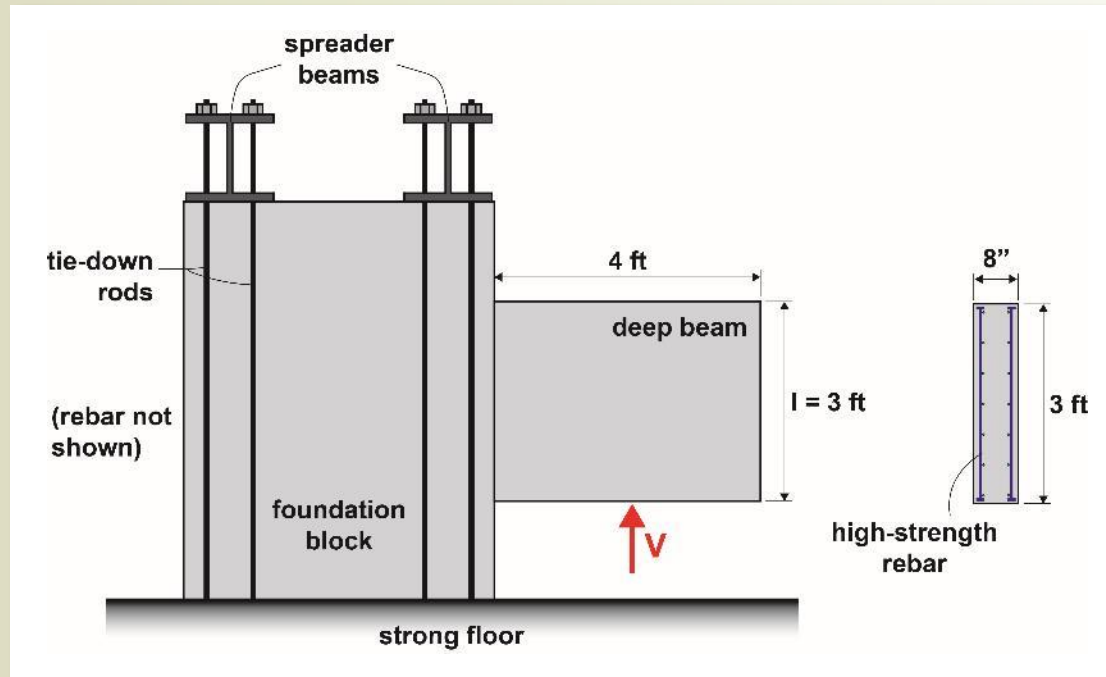
- “Generic wall” dimensions determined using publicly-available design control documents



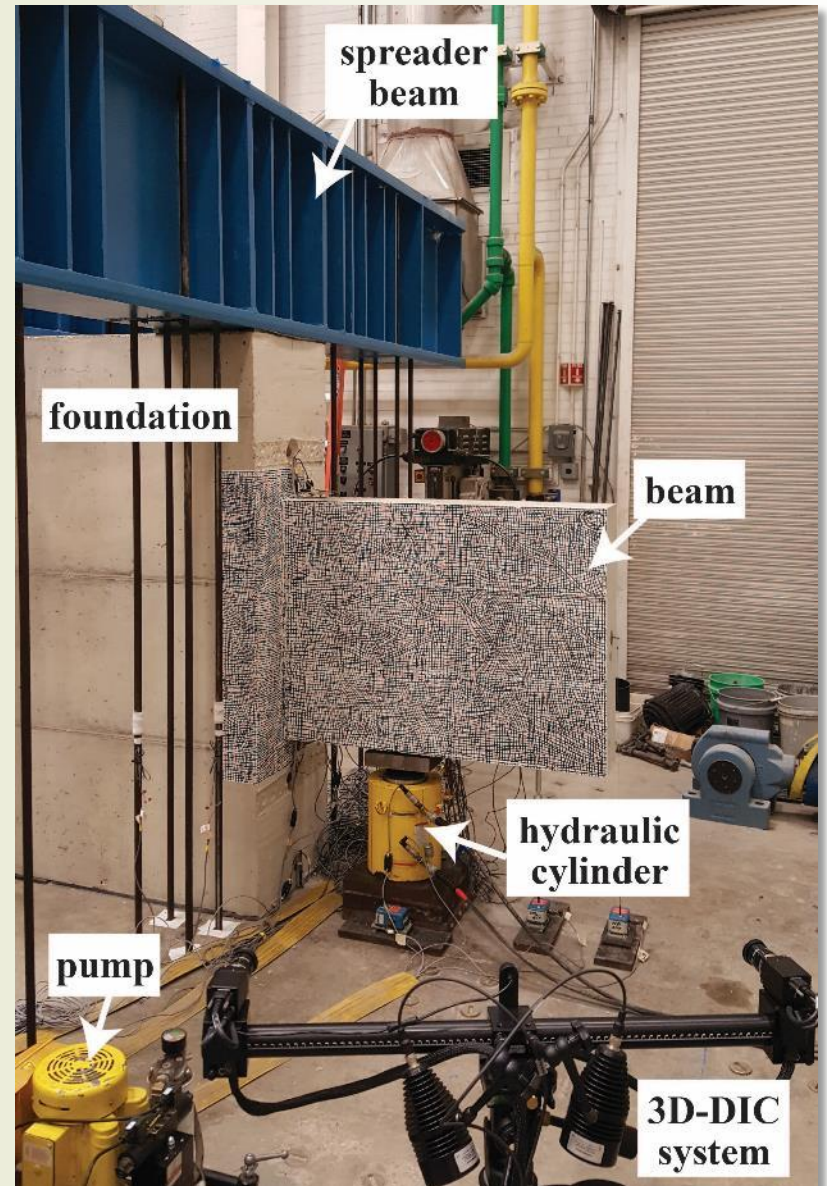
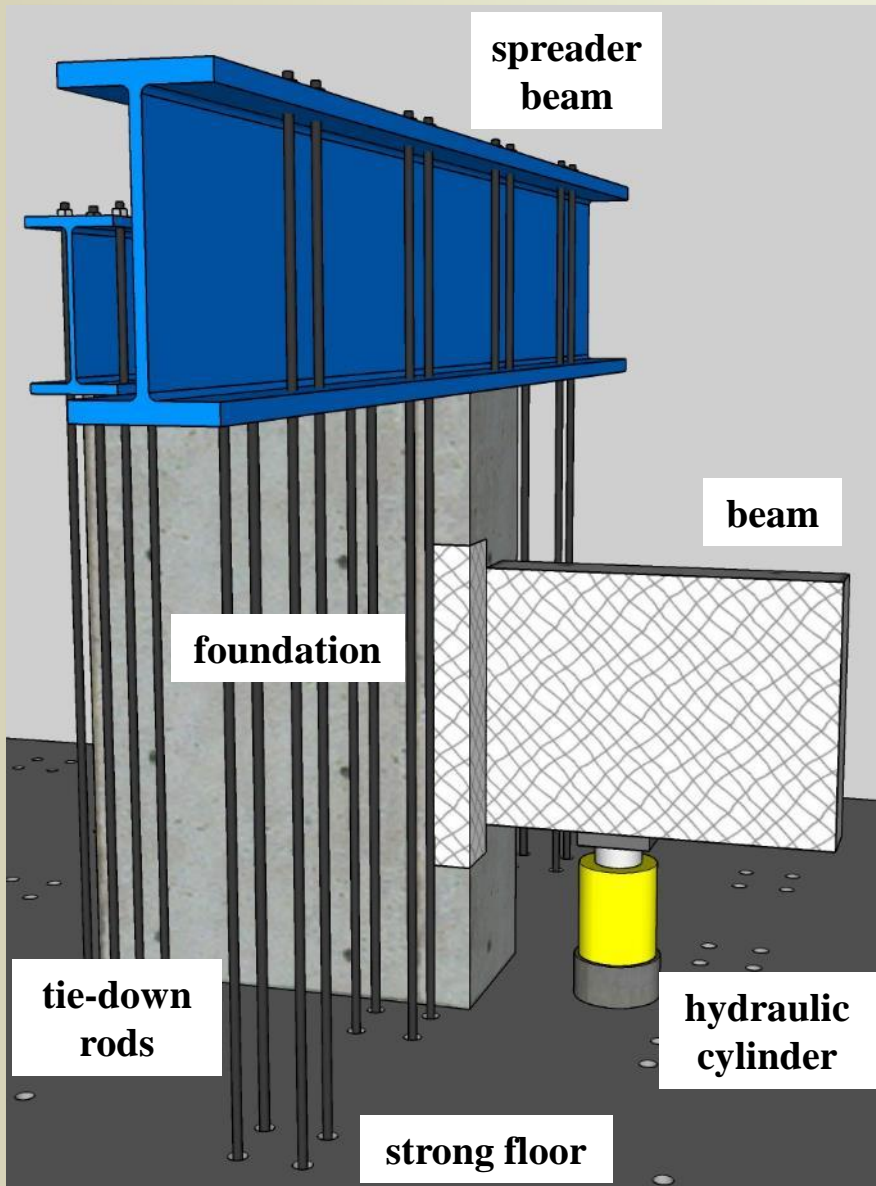
**representative slice of generic wall
for deep beam tests (@ 1:6.5 scale)**

4. Experimental Testing

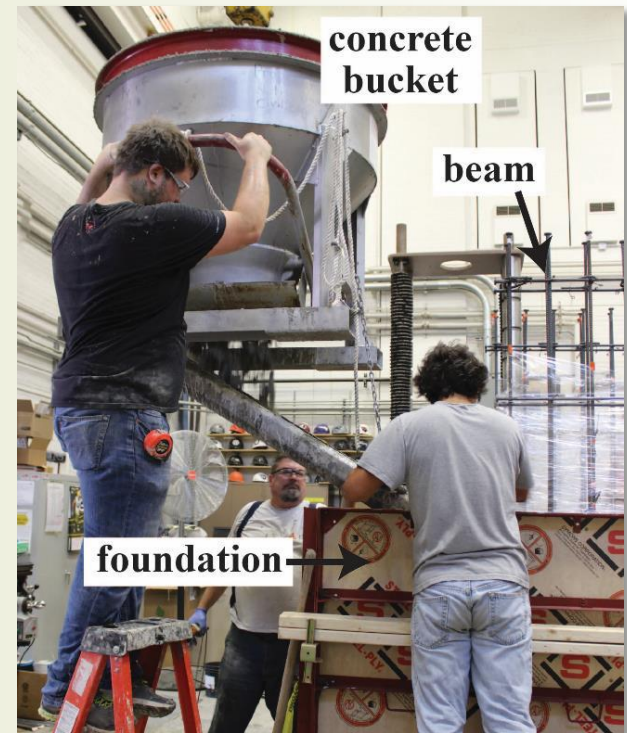
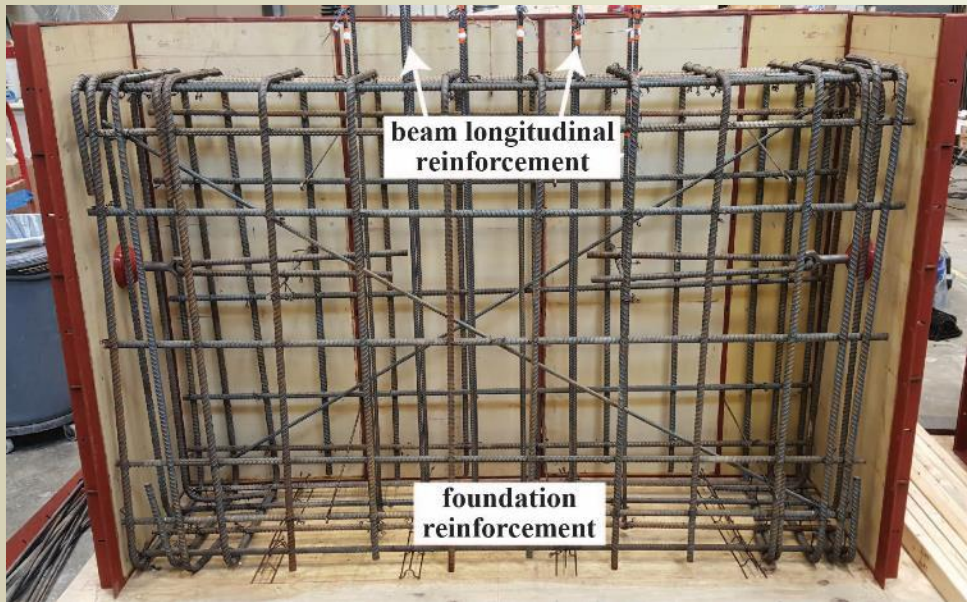
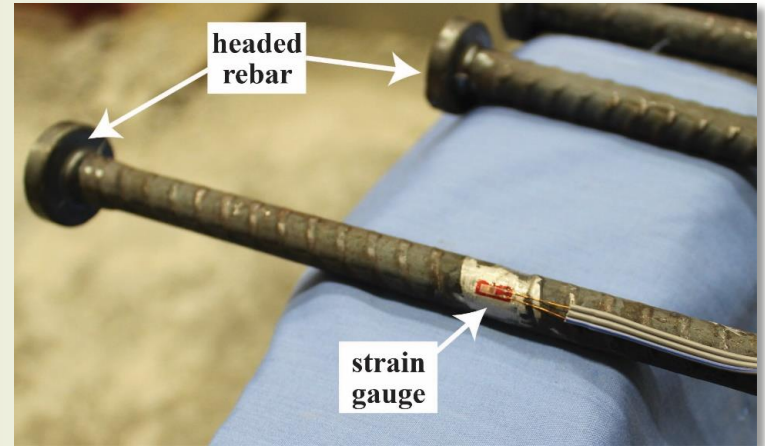
- “Generic wall” dimensions determined using publicly-available design control documents



4. Test Setup



4. Specimen Construction



4. Concrete Mix Design

Constituents	Normal-Strength Concrete	High-Strength Concrete
Portland Cement Type I/II (lb/yd ³)	182	400
Ground granulated blast-furnace slag (lb/yd ³)	437	350
Silica Fume (lb/yd ³)	0	50
Crushed Limestone (lb/yd ³) ^a	1745	1615
Fine Aggregate (lb/yd ³) ^a	1346	1353
Water (lb/yd ³) ^a	250	220
HRWR (fl. oz./cwt)	2.00	6.75
Water/Binder Ratio	0.41	0.28
Air Content	2.6%	1.5%
Slump (in.)	8	8.75
Measured 28-day f'_c (psi)	6500	14960
Predicted Temp. Rise (°F)	85	110

^aWeights of aggregates and water reported as saturated surfaced dry weight and weight of water above SSD, respectively.

4. Concrete Mix Design



Normal-Strength Concrete

$f'_c = 6500$ psi

slump = 8 in.



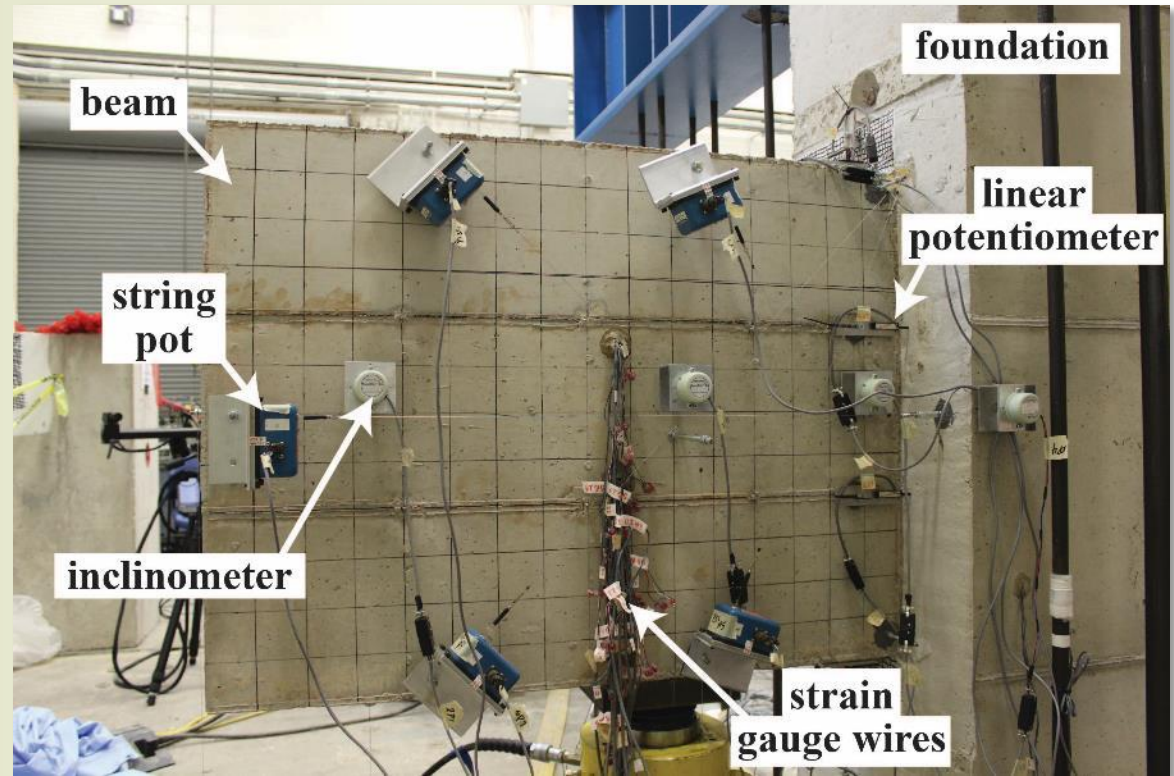
High-Strength Concrete

$f'_c = 14960$ psi

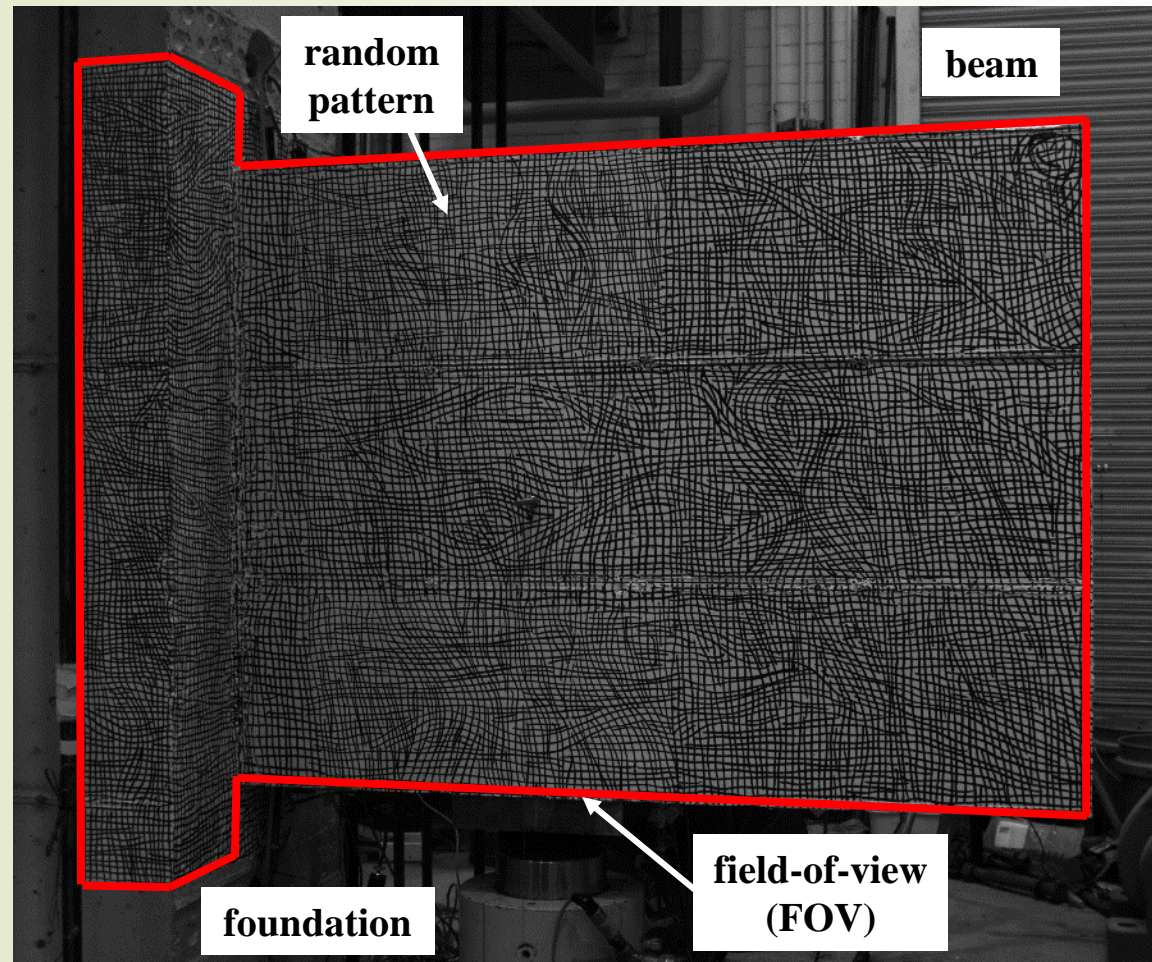
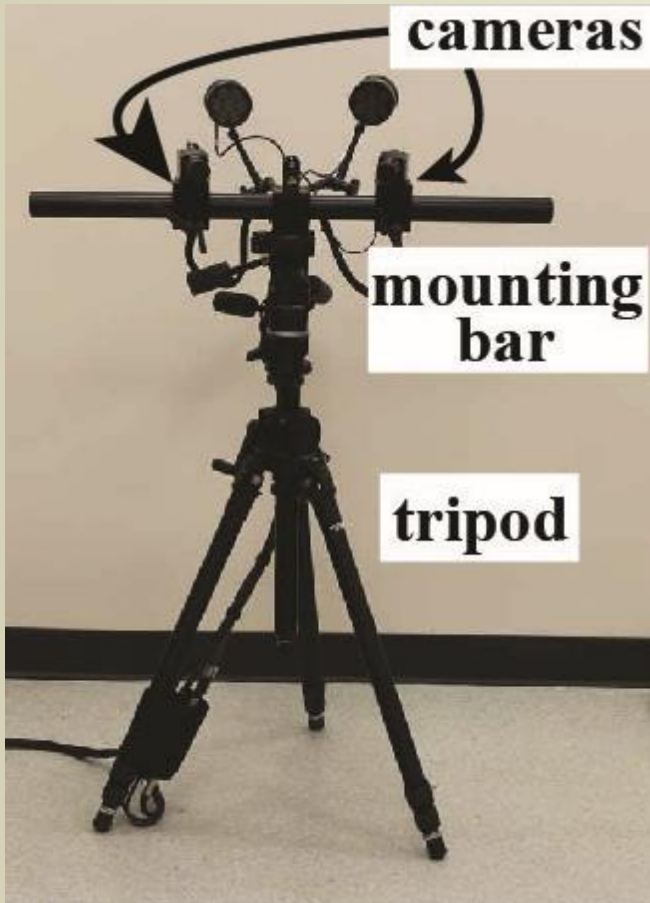
slump = 8.75 in.

4. Conventional Instrumentation

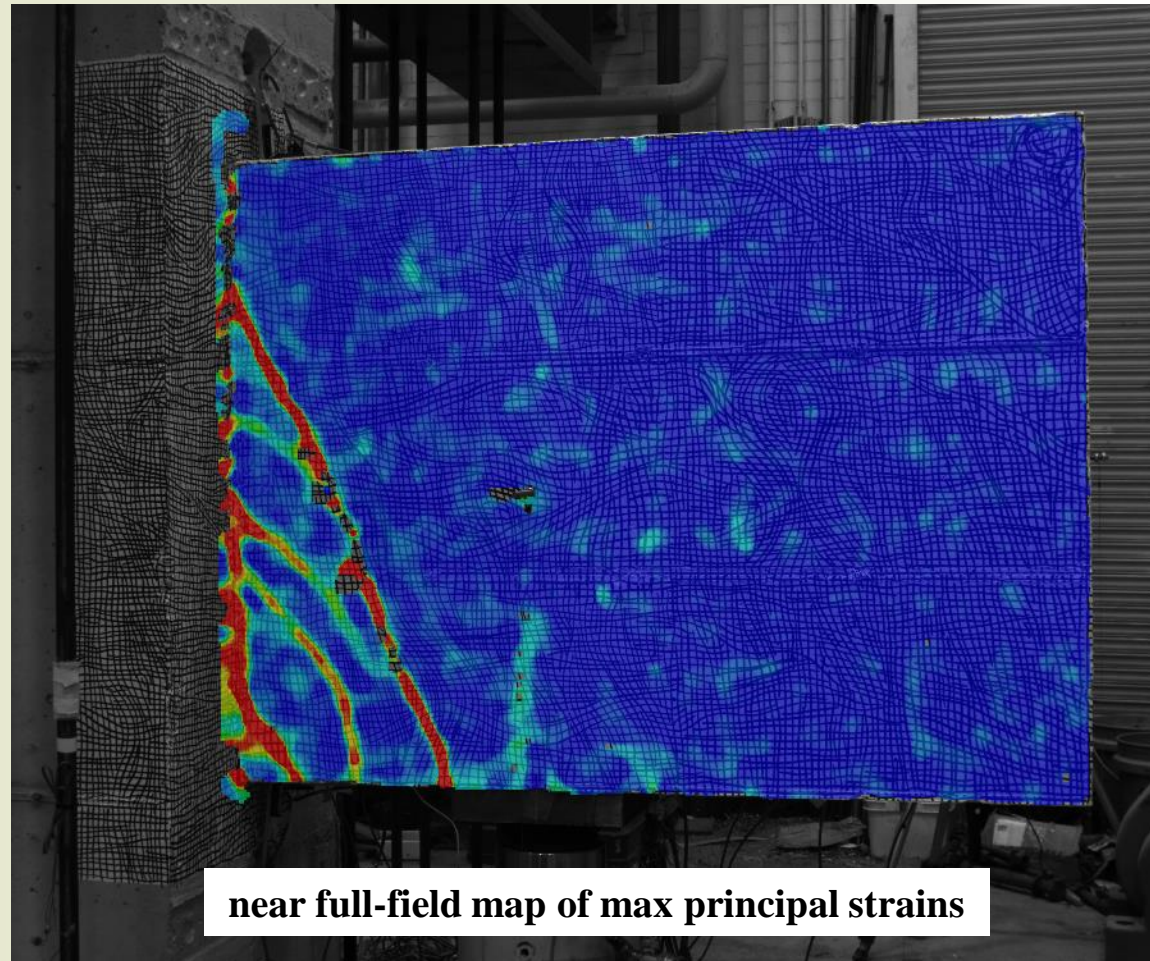
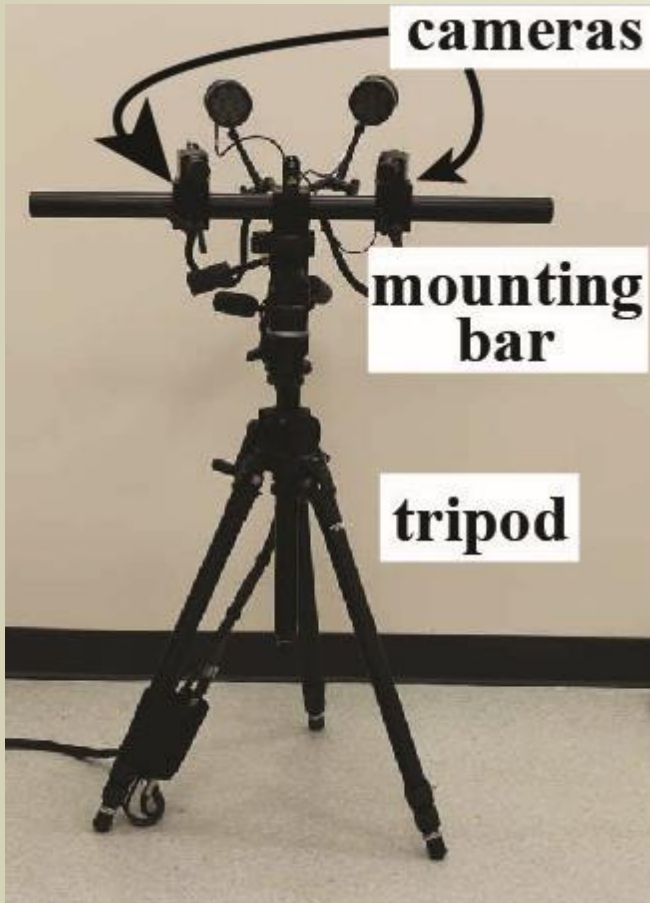
Type	Number
pressure transducer	2
string potentiometer	9
linear potentiometer	8
tiltmeter	4
strain gauge	42
TOTAL	65



4. 3D Digital Image Correlation



4. 3D Digital Image Correlation



4. Test Parameters to Date

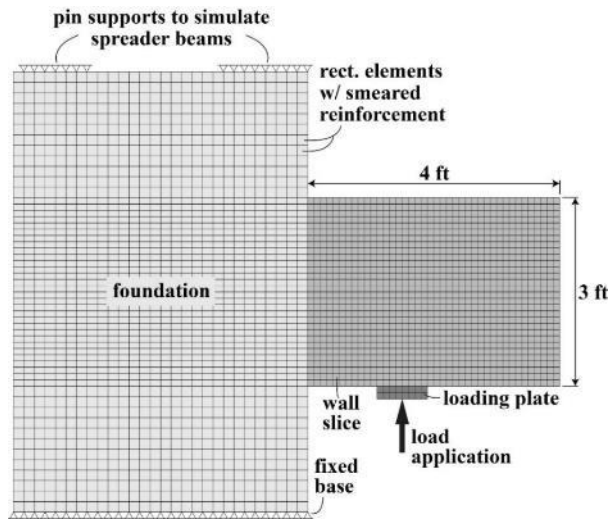
Specimen	f'_c (psi)	f_y (ksi)	ρ_s (%)	$M/(Vl_w)$
DB1	6500	70	0.833	0.5
DB2	6500	133	0.833	0.5
DB3	14960	70	0.833	0.5
DB4	14960	133	0.833	0.5

Definitions: f'_c – concrete 28 day compressive strength

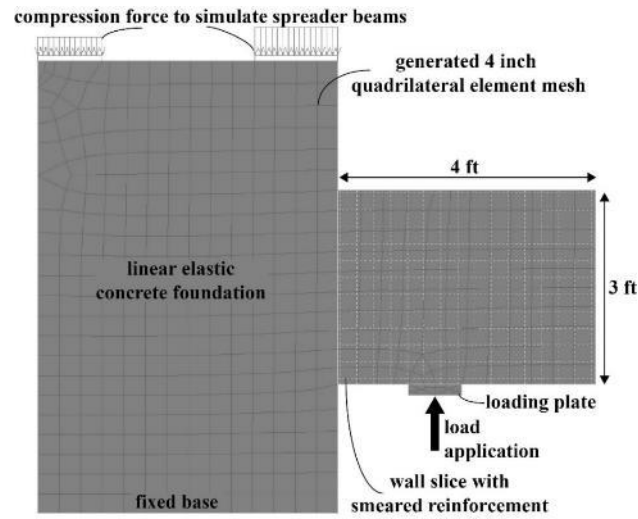
f_y – rebar yield strength, determined by tensile tests and 0.2% offset method

ρ_s – reinforcement ratio, symmetric for longitudinal and transverse rebar

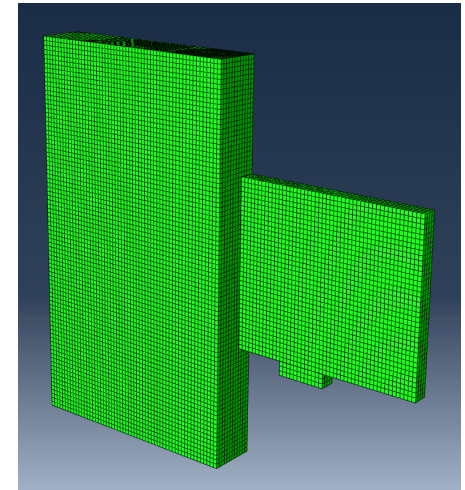
4. Pre-test Analyses



VecTor2

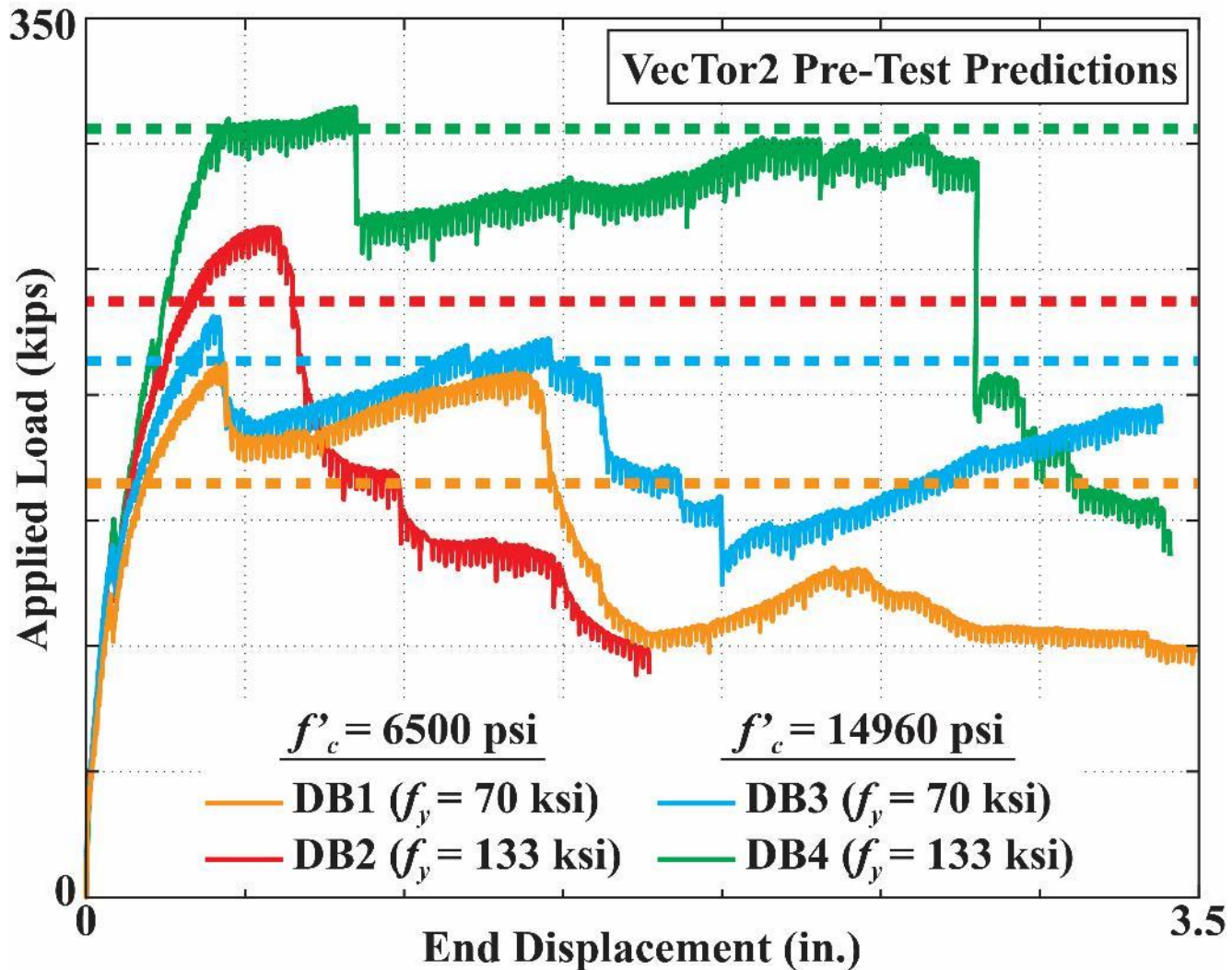


ATENA

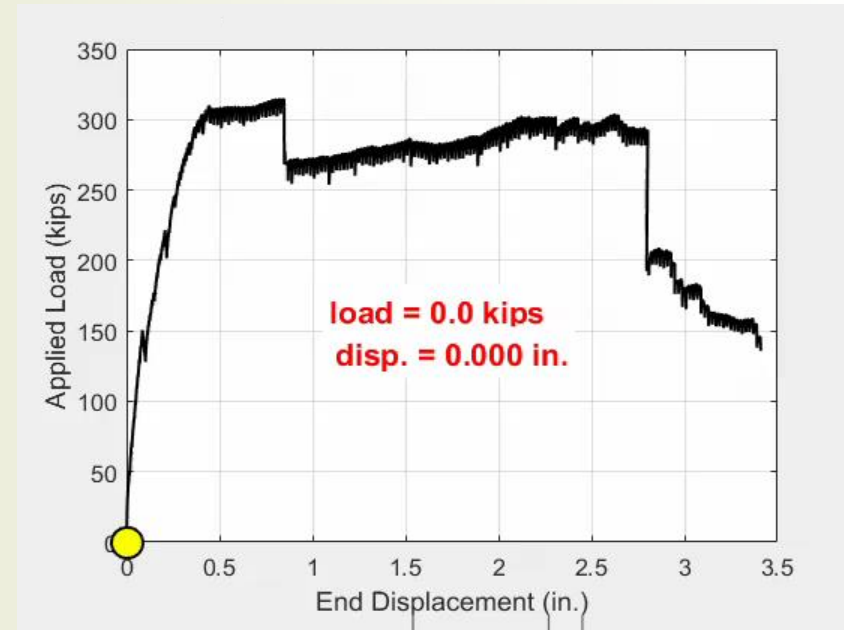
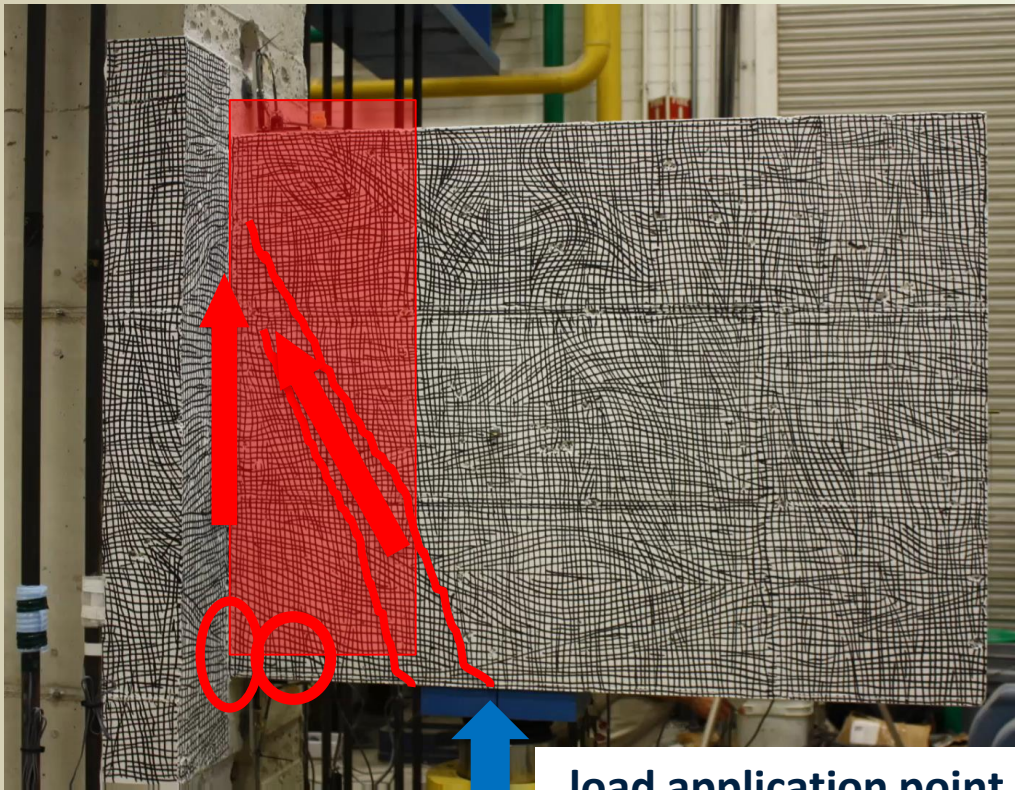


ABAQUS

4. Specimen Response

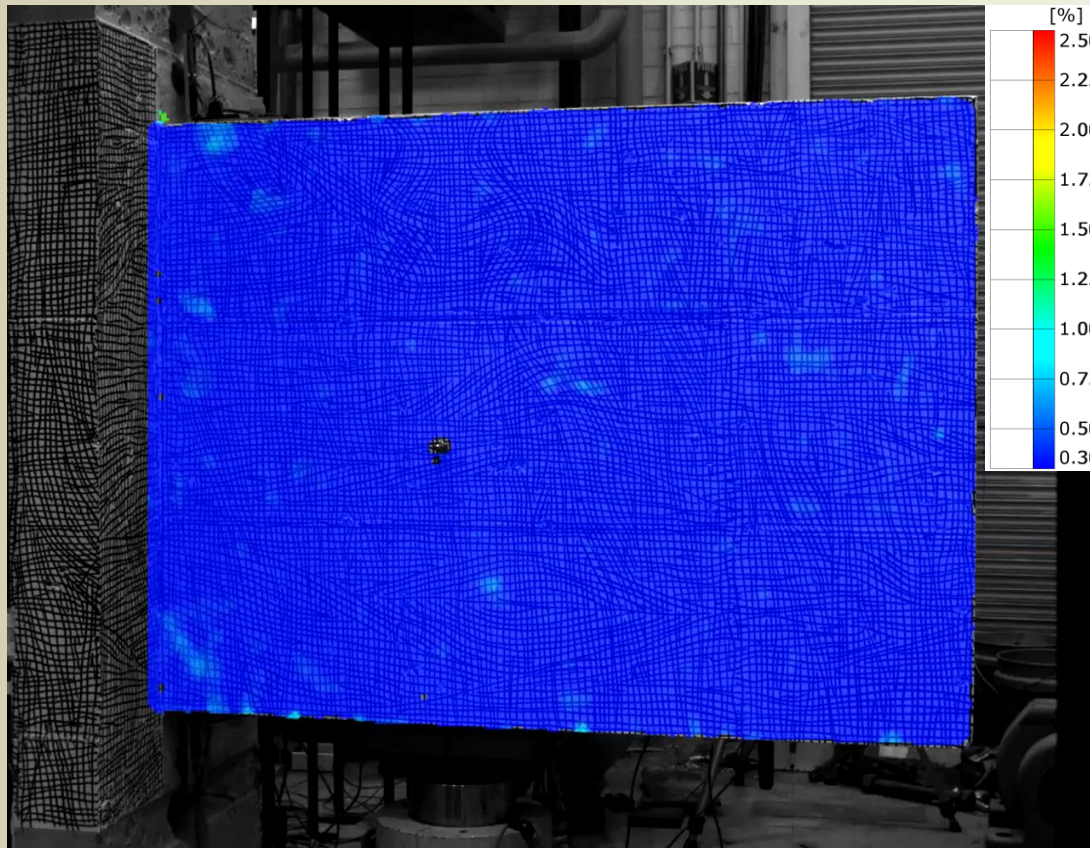


4. DB4 ($f'_c = 14960$ psi, $f_y = 133$ ksi)



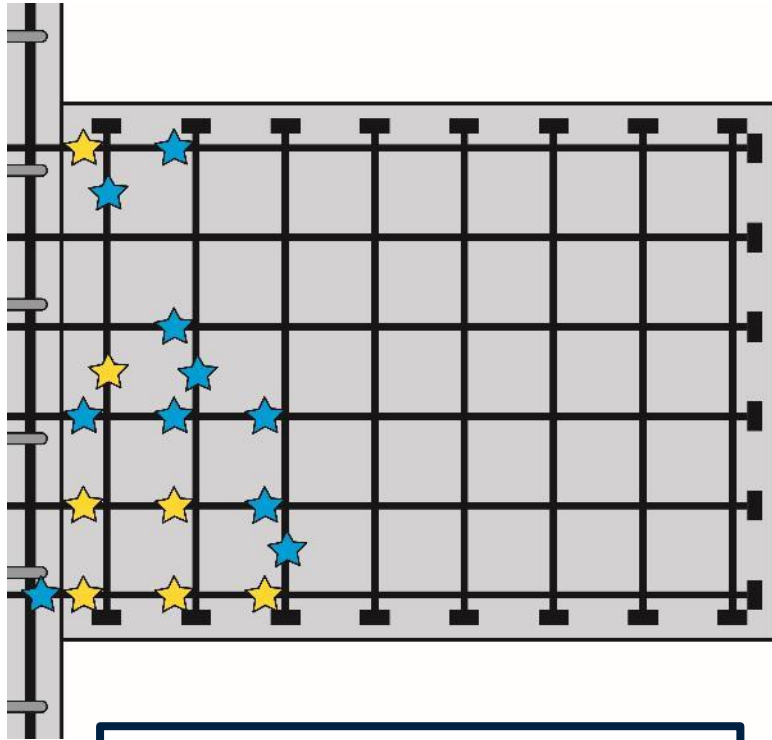
VIDEO, contact ykurama@nd.edu or athrall@nd.edu for more information

4. DB4 ($f'_c = 14960$ psi, $f_y = 133$ ksi)

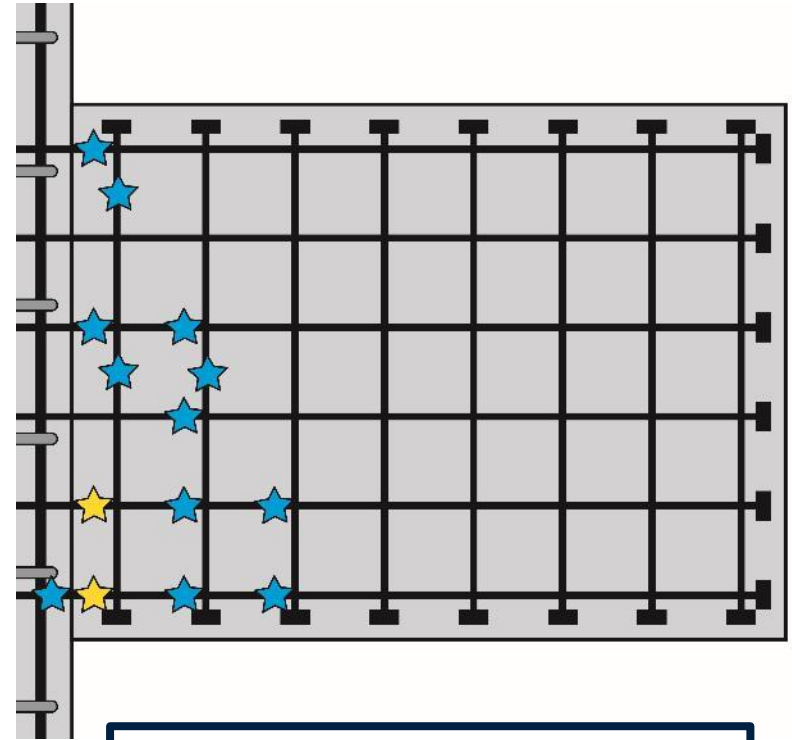


VIDEO, contact ykurama@nd.edu or athrall@nd.edu for more information

4. Strain Comparisons



DB4 $f'_c = 14960\text{psi}$ $f_y = 133\text{ksi}$



DB2 $f'_c = 6500\text{psi}$ $f_y = 133\text{ksi}$

- ★ active tension strain
- ★ tension yield ($6.85\text{ m}\epsilon$)

High-strength concrete able to better utilize higher yield strengths of reinforcement

4. Summary of Tests

- Most significant strength increase and most ductile failure for deep beams was when high-strength materials were used together (DB4)
- Isolated increase in rebar yield strength (DB2) resulted in higher increase in deep beam strength than isolated increase of concrete compressive strength (DB3)
- Pre-test analyses provided reasonable and conservative predictions for all specimens

Conclusions

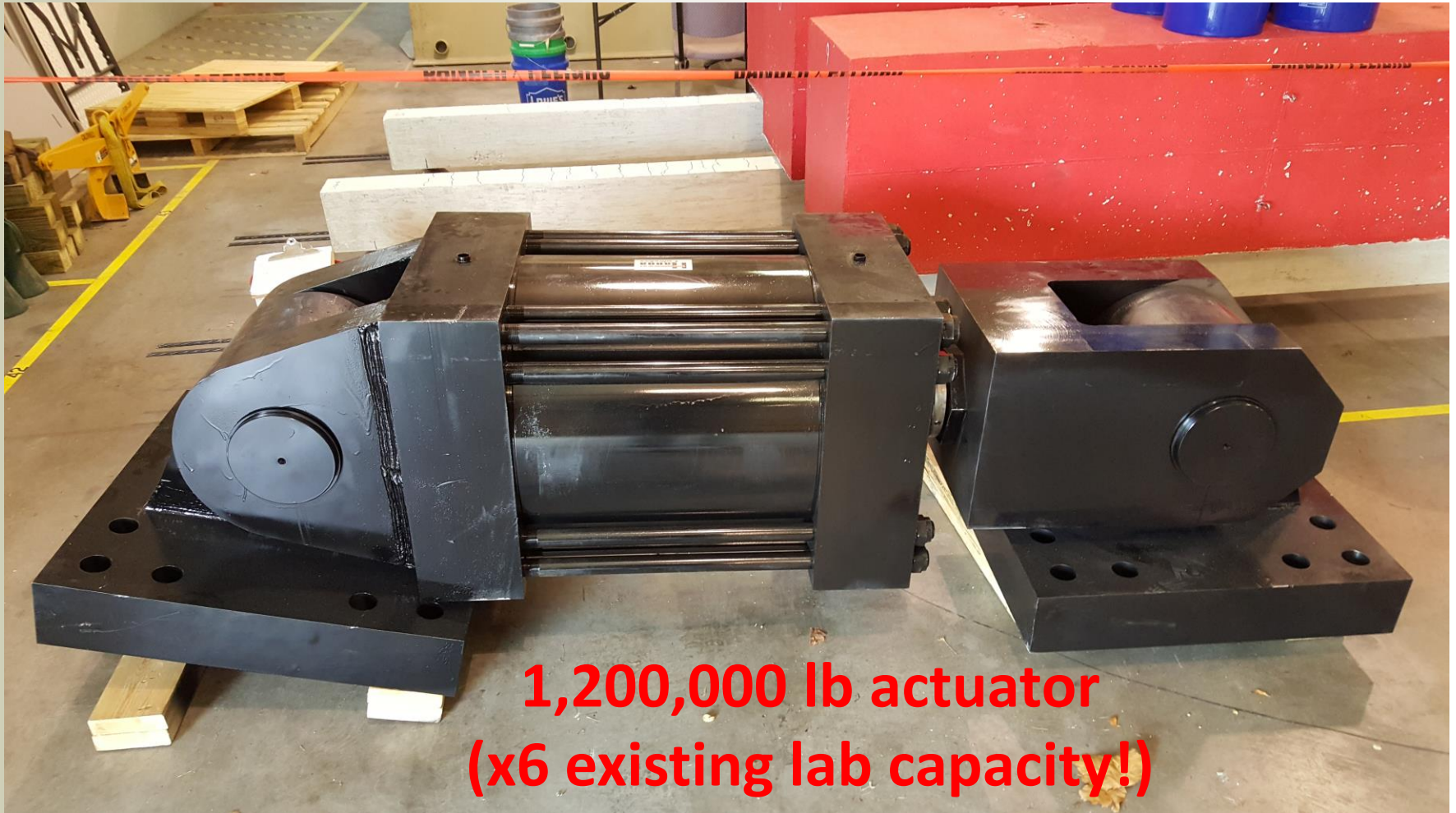
- High-strength steel more effective when combined with high-strength concrete
 - Numerically demonstrated (economics and peak strength)
 - Measured experimentally
- Greatest benefit for walls with low base moment to shear ratios and large reinforcement amounts; typical of nuclear concrete shear walls
- Largest economic and structural benefits when using Grade 100 rebar together with 10 ksi compressive strength concrete



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Future Shear Wall Tests



**1,200,000 lb actuator
(x6 existing lab capacity!)**

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 - Dayton Superior Corp.
 - HRC, Inc.
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Questions?



<http://phsrc-nuclearwalls.nd.edu>

